

EASTCOAST TESTING & ENGINEERING, INC.

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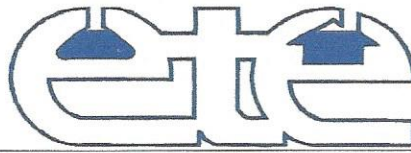
OCTOBER 22, 2017

**REPORT OF GEOTECHNICAL EXPLORATION &
ENGINEERING ANALYSIS - RECOMMENDATION
FOR**

JIM DEL VECCHIO, (GEC, INC.)

PROPOSED NEW BUILDINGS

**2681 SW 7th STREET
FORT LAUDERDALE, FLORIDA
BROWARD COUNTY, FLORIDA**



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October 22, 2017

Report of Engineering Evaluation for: Jim Del Vecchio, (GEC, Inc.)

Project: Proposed Buildings - Vacant

Location: 2681 SW 7th Street
Fort Lauderdale, Florida
Broward County, Florida

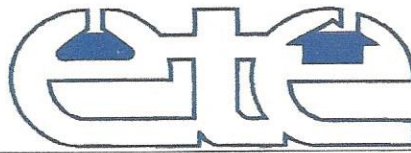
To Whom it May Concern;

As per your request EastCoast Testing & Engineering, Inc. performed the standard penetration test borings at the above reference location as requested. The purpose of this investigation was to provide information concerning the site and subsurface conditions in order to provide site preparation and foundation design recommendations for support of the proposed construction. This report presents our findings and foundation recommendations.

We understand that plans and information with regards for this project consist of two, (2) new one story buildings, (1564' square each) consisting of reinforced concrete masonry units, conventional concrete and wood truss. Elevations for the test borings were not provided at the time of our subsurface exploration. Finished floor elevations were not furnished, however we assume the ff to be a minimum of 18" above the lowest road crown. Major intersections for this project location are north of Davie Boulevard and east of Riverland Road in Fort Lauderdale, Florida, Broward County, Florida.

STANDARD PENETRATION TEST BORINGS

The test boring locations were determined by our drill supervisor and are indicated on the attached test boring report logs, and site plan. A review of our test boring indicates the upper level of subsoils are comprised of fine-grained sands with little/traces of limestone fragments, concrete debris, root, and silt, (topsoil) to a depth of approximately +/-1.0-2.5 feet below grade. Underlying this upper layer our borings discovered stratum of fine-grained sands in a loose to medium dense state of relative consolidation which continued to +/-7.0-8.0 feet below the existing surface grade elevation. From this depth were fine-medium grained sands with traces of silt primarily in a medium dense consolidated condition which extended to -15.0 feet. These medium dense sand substratum terminated our subsurface exploration at fifteen feet, maximum penetration.



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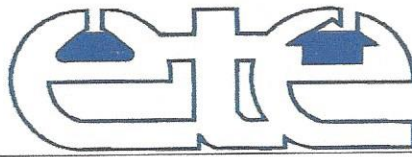
The results of our subsurface study confirm that the site is favorable for shallow foundations using geotechnical considerations. Existing subsurface conditions can support construction designed for support for a net allowable soil bearing capacity of 2500 pounds per square foot after "de-grubbing", backfilling and compaction. The proposed structure can be supported on shallow foundations. Detailed recommendations follow in the remaining sections of the report.

In order to prepare the site to support new construction designed for a net allowable soil bearing capacity of 2500 psf, we recommend the following procedures be implemented.

1. The proposed building pad areas and all new foundation elements shall be cleared and grubbed to a minimum depth of +/-1.0-2.0 feet plus a horizontal distance of five feet outside the perimeter. Care should be taken so as to insure the complete removal of all deleterious materials encountered including any topsoil, construction debris, asphaltic concrete, concrete, organics, grass, rubble, vegetation, stumps, roots, foreign material, debris, silts, clays or muck.

2. Once this has been inspected and approved by the engineer, compact the bottom of the excavated area with twenty, (perpendicular) overlapping passes in each of two directions with a large single smooth drum static/vibratory roller, (10-15 metric tons or larger) until the bottom 24" of the excavation have been compacted in excess of 98% of the materials modified maximum dry density as per ASTM D-1557. Field density-moisture tests and/or cone penetrometer tests shall be performed to verify the resultant compaction effort prior to the next phase of construction. Densification shall continue until no further settlement can be visually discerned at the subgrade surface. Each pass of the roller should overlap the preceding pass by one half of the width of the vibratory drum. Maximum speed of the roller should be two feet per second. For optimum compaction efficiency we recommend that the soils be within +/-2% of optimum moisture at the time of densification.

3. Once this has been achieved and verified by a geotechnical representative from this office or by the project engineer the proposed building pad areas may be brought to construction grade utilizing clean granular fills. The construction fill may be placed in lifts not to exceed twelve inches in compacted thickness. Each lift should be compacted to a minimum density of 98% of the materials modified proctor density as per ASTM D1557. Clean granular fills shall be construed to mean granular material containing no more than 5% by weight organic and clayey matter and no man-made debris. It shall be free of vegetation, foreign material, roots, fiber, branches, leaves, and should not contain any rock and gravel larger than 3 inches or 50% of the compacted layer thickness.



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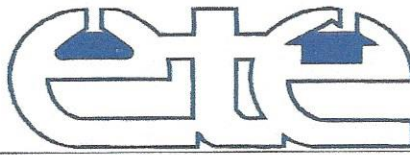
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4. Suitable materials for use as backfill consist of the following; GW, (well-graded gravel, and well-graded gravel with sand) GP, (poorly graded gravel and poorly graded gravel with sand) SW, (well-graded sand) or SP, (poorly graded sand). Suitable materials excavated and stockpiled at the site may be reused for backfill within the foundation after tested for approval by the engineer. Following and during the clearing and excavation stage the area shall be witnessed by an inspector from this laboratory or by the engineer for approval prior to in-situ densification. The excavated surface and each 12" compacted lift of on-site or imported fill material within the footing and slab areas shall be tested to within 98% of the Soils Modified Maximum Dry Density as per ASTM D1557 and verified with field density-moisture relationships. No vibration shall take place in the vicinity of any existing structure as this may cause localized damage to nearby structures.

Footing embedment shall be of sufficient depth below the adjacent grade so as to comply with all local and area building codes. Minimum footing widths of 18" and 36" are recommended for continuous wall footings and individual column pads respectively, although they may not develop the full allowable bearing pressures. To assure a safety factor against bearing capacity failure, the foundation bottoms should be placed a minimum of 18 inches below the lowest adjacent grade. Foundation elements may be designed as isolated footings or as a monolithic type of foundation/slab system, as long as ample consideration is given to the increased shear stresses inherent in monolithic systems at the slab to footing interfaces. Surface compaction specifications shall be verified utilizing in place density tests at the frequency of one test per 100 lf of wall footing, and one test per column pad. Slab areas and undisturbed pad lifts may be tested at the frequency of two tests per 2500 sf., but in no case less than three per lift. Reinforcement for footings should be proportioned to meet the requirements of ACI 318, latest edition, and other applicable codes.

Slab-on-grade construction may be employed for the ground floor of the structure. Ground floor slabs can bear directly on the densified structural fill after the backfill and compaction operations have been completed. Proper joints shall be provided at the junctions of the slabs with the building walls and columns so that a small amount of independent movement can occur without causing damage. A modulus of subgrade value of 120 pounds per cubic inch (pci) may be used for ground floor slab design.

Foundations designed and constructed in accordance with the recommendations of this report are expected to sustain a maximum total settlement of 1 inch and differential settlements between adjacent footings of not more than 1/2 inch or approximately one-half of the total settlement. Distortions that occur along the wall footings due to differential settlement should not be more than one in 500.



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An impervious membrane should be installed between the underside of the floor slab and the soil substrate to serve as a barrier to moisture rises from the subgrade. We recommend a minimum 10-mil thick film of polyethylene be utilized as a moisture barrier on this project. However, some floor coverings may have a comparatively sensitive tolerance to moisture flux that a thin, (10 ml) polyethylene film cannot suppress. Under these conditions, other types of moisture membranes may need to be considered.

The natural ground water table was found at +/-4'6"-5'0" at the time of our subsurface exploration. Fluctuation, however in the groundwater levels should be expected due to rainfall variations, seasonal climatic changes, construction activity and other on-site specific factors.

CONSTRUCTION PLAN & SPECIFICATIONS REVIEW

It is recommended that this office be provided the opportunity to make a general review of the foundation and earthwork plans and specifications prepared from the recommendations presented in this report.

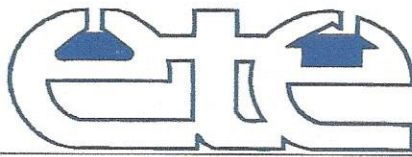
Our report has been written in a guideline recommendation format and is not appropriate for use as a specification-type format. It is recommended that this report not be made a part of the contract documents, however, it should be made available to prospective contractors for information purposes.

CONSTRUCTION RELATED SERVICES

We recommend the owner retain Eastcoast Testing & Engineering, Inc. to perform construction materials testing and observations on this site. Field tests and observations include foundation and pavement subgrades by performing quality assurance testing on the placement of compacted structural fills, and pavement courses. We can also provide concrete testing, pavement section testing, structural steel testing, general construction observation services, and Special Inspection services.

LIMITATIONS

Our geotechnical exploration study has been performed, our findings obtained, and our recommendations prepared in accordance with generally accepted geotechnical engineering conclusions and practices. EastCoast Testing & Engineering, Inc., (ETE) is not responsible for any independent conclusions, opinions or recommendations made by others based on the data contained in this report.



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This report does not reflect any variations which may occur away from the soil borings. The discovery of any subsurface conditions which deviates from the data obtained during this geotechnical investigation should be reported to us for further analysis and evaluation.

The Standard Penetration Test ASTM D-1586

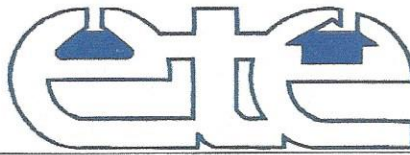
The Standard Penetration Test is the most commonly employed tool utilized to identify in-situ subsurface soil conditions. The "N" values obtained from the boring provide an accurate estimation of internal soil characteristics such as relative density, internal shear strength, angle of internal friction, and the approximate range of the soil's unit weight. These "N" values represent the resistance of a 2-inch diameter split spoon sampler driven by a 140-pound hammer free falling 30 inches. Each drive of the 24 inches long split spoon is divided into four six-inch increments. The second and third increments are totaled to produce the "N" value found on your report.

The Standard Penetration Test also allows for the recovery of soil samples which are returned to our laboratory and visually examined and classified. The SPT samples are available for laboratory testing if requested. Samples are generally held for 30-90 days unless otherwise directed by the client.

An approximate ground water table is obtained from the borehole upon completion of the drilling procedures. This water table is useful in the general evaluation of particular soil conditions, and may give the contractor some insight into what can be anticipated during construction. It should be noted that the ground water level will fluctuate seasonally. This level may also be affected by local draw-downs, soil conditions, and the watersheds contribution to the underlying aquifer. It should not be construed to be a measure of the soil's permeability, or of the de-watering characteristics of the site.

Although the standard penetration test is one of the most reliable methods used to identify soil characteristics and types, it may only represent a small fraction of the materials actually deposited at the site. As is common industry practice, we have assumed a uniformity of a profile between borings to provide a subsoil profile for engineering purposes. This profile is strictly based on the data obtained from the borings, and if unusual or varying conditions are found we should be notified immediately.

A test is expressly representative of the immediate location tested, and the reliability of the conclusions is a direct result of the quantity of tests performed. Any variation in location may reveal similarly some changes in the depth, thickness, texture, and conditions of the stratum encountered.



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Unless specifically stated otherwise, and specifically directed and prearranged by the client, all elevations are taken with respect to the existing ground surface at the time of testing. Boring locations are usually obtained in the field by pacing off distances and approximating right angles to landmarks and property corners. More precise locations may be obtained from on site surveys and placement of the boring locations by a Land Surveyor, Registered in the State of Florida. These services are provided at additional costs and are beyond the scope of this report.

The data presented herein was obtained for the specific purposes stated in this report, and should not be misconstrued to apply to any other circumstance, project, or ancillary use unless so specified and addressed by the engineer of record.

Thank you for using EASTCOAST TESTING AND ENGINEERING, INC., for your geotechnical needs. Should you need further assistance with this or any other project, please contact this office.

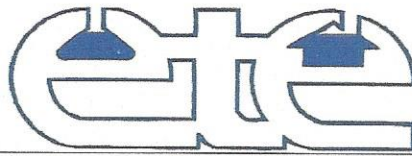
Respectfully Submitted;
EASTCOAST TESTING & ENGINEERING, INC.
Certification of Authorization #3425


Mohammed A. Hai, P.E.
Senior Geotechnical Engineer
Florida Registration No. 59345

10/22/2017


Craig Smith, President

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TEST BORING REPORT

LABORATORY NUMBER: **171676-A**

BORING NUMBER: **1**

CLIENT: **JIM DEL VECCHIO**

CUSTOMER #: _____

PROJECT: **PROPOSED BUILDINGS, (2) @ 1564' SQUARE**

REW CHIEF: **H.E.**

PROJECT ADDRESS: **2681 SW 7th STREET - FORT LAUDERDALE, FLORIDA**

DRILLER: **W.L.**

BORING LOCATION: **~35' N. & 18' E. OF THE SW PROPERTY CORNER**

DRILL RIG#: **F350**

GROUND WATER: **4'8"** DATE: **10/18/17** ELEV: **N/F**

CASING: **3"**

NOTE: SURVEY NOT GIVEN UNLESS NOTED: B.E.G: BELOW EXISTING GRADE LOCATIONS: APPROX UNLESS STAKED

DEPTH FEET	SAMPLE NUMBER	BORING NUMBER: 1	PAGE NUMBER: 1	VISUAL SOIL CLASSIFICATION/AASHTO M145/ASTMD2487	DEPTH	N VALUES		SPT BLOWS PER 6"	
1	1	BROWN-GRAY FINE-GRAINED SAND LITTLE LIMESTONE & ROOT, (TOPSOIL)	0.0"-2.5'			1	1	1	1
2	2					1	1		
3						2	3		
4	2	LIGHT BROWN-GRAY FINE-GRAINED SAND, (SP)	2.5-8.0'			7	4	5	5
5						6	6		
6						12	6	4	
7						3	4		
8						10	6	6	
9	3	DARK BROWN FINE-MEDIUM GRAINED SAND TRACE SILT, (SP)	8.0-10.0'			8	8	11	11
10						25	14	14	
11									
12									
13									
14									
15									
16									

BOTTOM OF BORING @ 10.0 FEET

STANDARD PENETRATION TEST BORING:

BLOWS PER FOOT ON 2" O.D. SAMPLER WITH A 140 LB. HAMMER FALLING 30"

SOIL INVESTIGATION & SAMPLING BY AUGER BORINGS: A.S.T.M. D 1452/STANDARD PENETRATION TEST: ASTM D1586. THE SAMPLES COLLECTED CONSTITUTE A MINUTE PERCENTAGE OF THE SUBSOILS AT THE SITE. AS A MUTUAL PROTECTION THE SOILS WILL BE STORED IN OUR LABORATORY FACILITIES FOR A MAXIMUM OF THREE (3) MONTHS. THE OWNER, ARCHITECT AND/OR ENGINEER ARE ENCOURAGED TO VISUALLY INSPECT SAMPLES PRIOR TO PURCHASE OF PROPERTY AND DESIGN OF THE STRUCTURE.

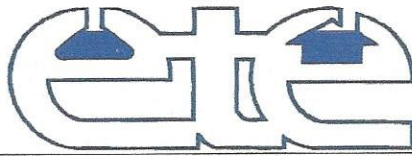
RESPECTFULLY SUBMITTED,
EASTCOAST TESTING & ENGINEERING, INC.,
CERTIFICATE OF AUTHORIZATION #3425

M. HAI 10/22/2017
MOHAMMED A. HAI
SENIOR GEOTECHNICAL ENGINEER
FLORIDA REGISTRATION NO. 59345

C. SMITH
CRAIG SMITH, PRESIDENT

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TEST BORING REPORT

LABORATORY NUMBER: 171676-B

BORING NUMBER: 2

CLIENT: JIM DEL VECCHIO

CUSTOMER #: _____

PROJECT: PROPOSED BUILDINGS, (2) @ 1564' SQUARE

REW CHIEF: H.E.

PROJECT ADDRESS: 2681 SW 7th STREET - FORT LAUDERDALE, FLORIDA

DRILLER: W.L.

BORING LOCATION: ~35' S. & 25' E. OF THE NW PROPERTY CORNER

DRILL RIG#: F350

GROUND WATER: 4'10" DATE: 10/18/17 ELEV: N/F

CASING: 3"

NOTE: SURVEY NOT GIVEN UNLESS NOTED: B.E.G: BELOW EXISTING GRADE LOCATIONS: APPROX UNLESS STAKED

DEPTH FEET	SAMPLE NUMBER	BORING NUMBER: <u>2</u>	PAGE NUMBER: <u>1</u>	VISUAL SOIL CLASSIFICATION/AASHTO M145/ASTMD2487	DEPTH	N VALUES		SPT BLOWS PER 6"	
1	1	BROWN-GRAY FINE-GRAINED SAND TRACE ROOT & SILT, (TOPSOIL)	0.0"-1.0'	LIGHT BROWN-GRAY FINE-GRAINED SAND, (SP)	1.0-7.0'	6	1	2	
2	2					4	6		
3						5	7		
4						13	6	6	
5						7	7	6	
6						7	1	2	
7						2	2	3	
8	3	VERY DARK BROWN-GRAY FINE-MEDIUM GRAINED SAND TRACE SILT, (SP)	7.0-10.0'		8	8	5	7	
9						6	9		
10						22	13	18	
11	4	DARK BROWN FINE-MEDIUM GRAINED SAND TRACE SILT, (SP)	10.0-15.0'		21	12	12	10	
12						11	11	13	
13						11	12	6	
14						11	5	5	
15						8	8	10	
16						BOTTOM OF BORING @ 15.0 FEET			

STANDARD PENETRATION TEST BORING:

BLOWS PER FOOT ON 2" O.D. SAMPLER WITH A 140 LB. HAMMER FALLING 30"

SOIL INVESTIGATION & SAMPLING BY AUGER BORINGS: A.S.T.M. D 1452/STANDARD PENETRATION TEST: ASTM D1586. THE SAMPLES COLLECTED CONSTITUTE A MINUTE PERCENTAGE OF THE SUBSOILS AT THE SITE. AS A MUTUAL PROTECTION THE SOILS WILL BE STORED IN OUR LABORATORY FACILITIES FOR A MAXIMUM OF THREE (3) MONTHS. THE OWNER, ARCHITECT AND/OR ENGINEER ARE ENCOURAGED TO VISUALLY INSPECT SAMPLES PRIOR TO PURCHASE OF PROPERTY AND DESIGN OF THE STRUCTURE.

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M. A. H. 10/22/2017
MOHAMMED A. HA!
SENIOR GEOTECHNICAL ENGINEER
FLORIDA REGISTRATION NO. 59345

C. Smith
CRAIG SMITH, PRESIDENT

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TEST BORING REPORT

LABORATORY NUMBER: **171676-C**

BORING NUMBER: **3**

CLIENT: **JIM DEL VECCHIO**

CUSTOMER #: _____

PROJECT: **PROPOSED BUILDINGS, (2) @ 1564' SQUARE**

REW CHIEF: **H.E.**

PROJECT ADDRESS: **2681 SW 7th STREET - FORT LAUDERDALE, FLORIDA**

DRILLER: **W.L.**

BORING LOCATION: **~20' S. & 40' W. OF THE NE PROPERTY CORNER**

DRILL RIG#: **F350**

GROUND WATER: **4'6"** DATE: **10/18/17** ELEV: **N/F**

CASING: **3"**

NOTE: SURVEY NOT GIVEN UNLESS NOTED:

B.E.G: BELOW EXISTING GRADE

LOCATIONS: APPROX UNLESS STAKED

DEPTH FEET	SAMPLE NUMBER	BORING NUMBER: 3	PAGE NUMBER: 1	VISUAL SOIL CLASSIFICATION/AASHTO M145/ASTMD2487	DEPTH	N		SPT	
						VALUES		BLOWS PER 6"	
1	1			BROWN FINE-GRAINED SAND LITTLE LIMESTONE & ROOT,	0.0"-2.0'			1	3
2				(TOPSOIL)		9		6	7
3	2			BROWN FINE-GRAINED SAND & LIMESTONE FRAGMENTS, (SP)	2.0-3.0'			6	6
4	3			LIGHT BROWN-TAN FINE-GRAINED SAND, (SP)	3.0-5.0'	12		6	6
5								7	5
6	4			LIGHT BROWN-GRAY FINE-GRAINED SAND, (SP)	5.0-7.0'	9		4	4
7								5	7
8	5			VERY DARK BROWN-GRAY FINE-MEDIUM GRAINED SAND TRACE SILT,	7.0-10.0'	15		8	4
9				(SP)				4	4
10						10		6	6
11									
12									
13									
14									
15									
16									

BOTTOM OF BORING @ 10.0 FEET

STANDARD PENETRATION TEST BORING:

BLOWS PER FOOT ON 2" O.D. SAMPLER WITH A 140 LB. HAMMER FALLING 30"

SOIL INVESTIGATION & SAMPLING BY AUGER BORINGS: A.S.T.M. D 1452/STANDARD PENETRATION TEST: ASTM D1586. THE SAMPLES COLLECTED CONSTITUTE A MINUTE PERCENTAGE OF THE SUBSOILS AT THE SITE. AS A MUTUAL PROTECTION THE SOILS WILL BE STORED IN OUR LABORATORY FACILITIES FOR A MAXIMUM OF THREE (3) MONTHS. THE OWNER, ARCHITECT AND/OR ENGINEER ARE ENCOURAGED TO VISUALLY INSPECT SAMPLES PRIOR TO PURCHASE OF PROPERTY AND DESIGN OF THE STRUCTURE.

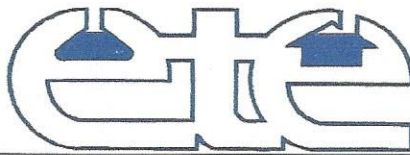
RESPECTFULLY SUBMITTED,
EASTCOAST TESTING & ENGINEERING, INC.,
CERTIFICATE OF AUTHORIZATION #3425

M. *AA ai* 10/22/2017
MOHAMMED A. HAI
SENIOR GEOTECHNICAL ENGINEER
FLORIDA REGISTRATION NO. 59345

C Smith
CRAIG SMITH, PRESIDENT

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THRESHOLD/SPECIAL INSPECTIONS, BORINGS, DENSITY, CONCRETE, ASPHALT, ETC, A "GEOTECHNICAL TESTING LAB"



EASTCOAST TESTING & ENGINEERING, INC.

4100 North Powerline Rd. - Suite G-1
Pompano Beach, Florida 33073

4361 Okeechobee Blvd. - Suite A-5
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Broward (954) 972.7645 (SOIL) - Dade (954) 972.7645

(954).972.SOIL

FACSIMILE #954-9718872

ESTABLISHED 1981

ETE@BELLSOUTH.NET

TEST BORING REPORT

LABORATORY NUMBER: 171676-D

BORING NUMBER: 4

CLIENT: JIM DEL VECCHIO

CUSTOMER #: _____

PROJECT: PROPOSED BUILDINGS, (2) @ 1564' SQUARE

REW CHIEF: H.E.

PROJECT ADDRESS: 2681 SW 7th STREET - FORT LAUDERDALE, FLORIDA

DRILLER: W.L.

BORING LOCATION: ~30' N. & 20' W. OF THE SE PROPERTY CORNER

DRILL RIG#: F350

GROUND WATER: 5'0" DATE: 10/18/17 ELEV: N/F

CASING: 3"

NOTE: SURVEY NOT GIVEN UNLESS NOTED:

B.E.G: BELOW EXISTING GRADE

LOCATIONS: APPROX UNLESS STAKED

DEPTH FEET	SAMPLE NUMBER	BORING NUMBER: <u>4</u>	PAGE NUMBER: <u>1</u>	DEPTH	N VALUES	SPT	
						BLOWS PER 6"	
1	1	VISUAL SOIL CLASSIFICATION/AASHTO M145/ASTMD2487		0.0"-1.5'	8	3	2
2		BROWN FINE-GRAINED SAND SOME/LITTLE CONCRETE DEBRIS, (TOPSOIL)				6	9
3	2	LIGHT BROWN FINE-GRAINED SAND, (SP)		1.5-5.0'	14	9	7
4						7	7
5						7	6
6	3	LIGHT BROWN-GRAY FINE-GRAINED SAND, (SP)		5.0-8.0'	10	4	3
7						4	6
8					14	8	6
9	4	DARK BROWN FINE-MEDIUM GRAINED SAND TRACE SILT, (SP)		8.0-15.0'		7	9
10					20	11	13
11						9	7
12					15	8	8
13						7	9
14					15	6	6
15						8	10
16		BOTTOM OF BORING @ 15.0 FEET					

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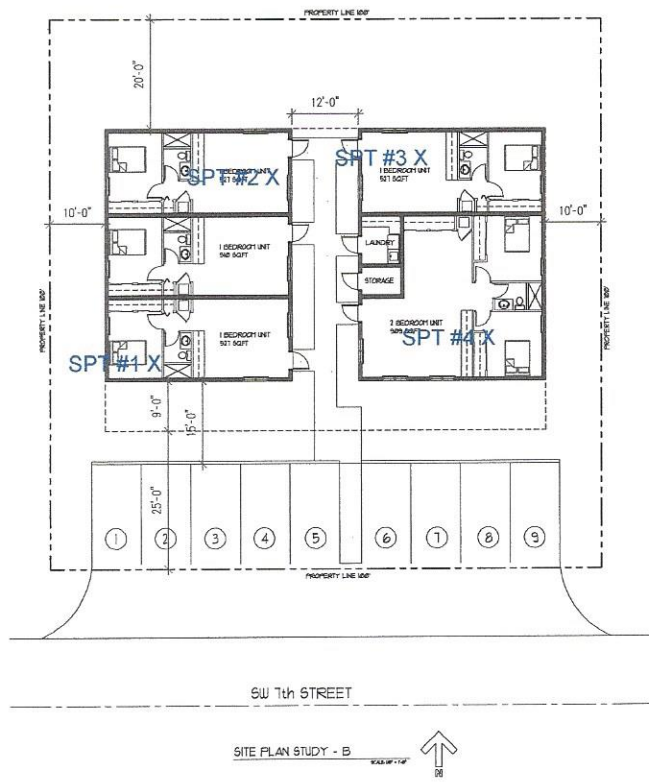
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SITE PLAN STUDY - B

LOT SIZE = 10,000 SQFT.

PARKING:
1.15 SPACES PER 1 BEDROOM UNIT
2 SPACES PER 2 BEDROOM UNIT

TOTAL 9 PARKING REQUIRED

15' LANDSCAPE BUFFER BETWEEN
PARKING & BUILDING

CITY ENGINEER APPROVAL REQUIRED
FOR BACK-OUT PARKING

400 SQFT. MINIMUM UNIT SIZE

2 BUILDINGS
EACH BUILDING 1564 SQFT.
TOTAL = 3,128 SQFT.

10FT. MINIMUM BETWEEN BUILDINGS

LEGAL DESCRIPTION:

LAST CHANGE VILLAGE #1-0 IS LOT #1, ACCORDING TO THE
PLAN THEREOF OF THE PUBLIC RECORDS OF DICKSON COUNTY,
FLORIDA.

SETBACK REQUIREMENTS

FRONT 25 FT.
SIDE 10 FT.
REAR 10 FT.

FLORIDA BUILDING CODE
5th EDITION (2014)

ZONING R1M1-25

SPT: X - STANDARD PENETRATION TEST, (BORING) - NTS

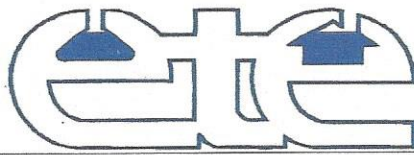
Architectural Design, Inc.
1811 E. Highway 10, P.O. Box 2000, Ft. Lauderdale, FL 33309
Tel: (954) 562-2222, Fax: (954) 562-2277
www.architectural-design.com

Professional Seal: Joviter F. Acebo, P.E., License No. 45822, State of Florida

Site Feasibility Study:
Jim Del Vecchio
2005 SW 7th Street
Fort Lauderdale, FL

Project: _____
Revisions: _____

Sheet Title: _____
Date: 11-15-2015
Drawn By: JA / MP
Sheet No.: A-2
Drawing 2 of 3



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PROPOSED BUILDINGS - 2 @ 1564' SQ.
2681 SW 7th STREET
FORT LAUDERDALE, FLORIDA

SITE LOCATION MAP

Legend

2681 SW 7th St



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